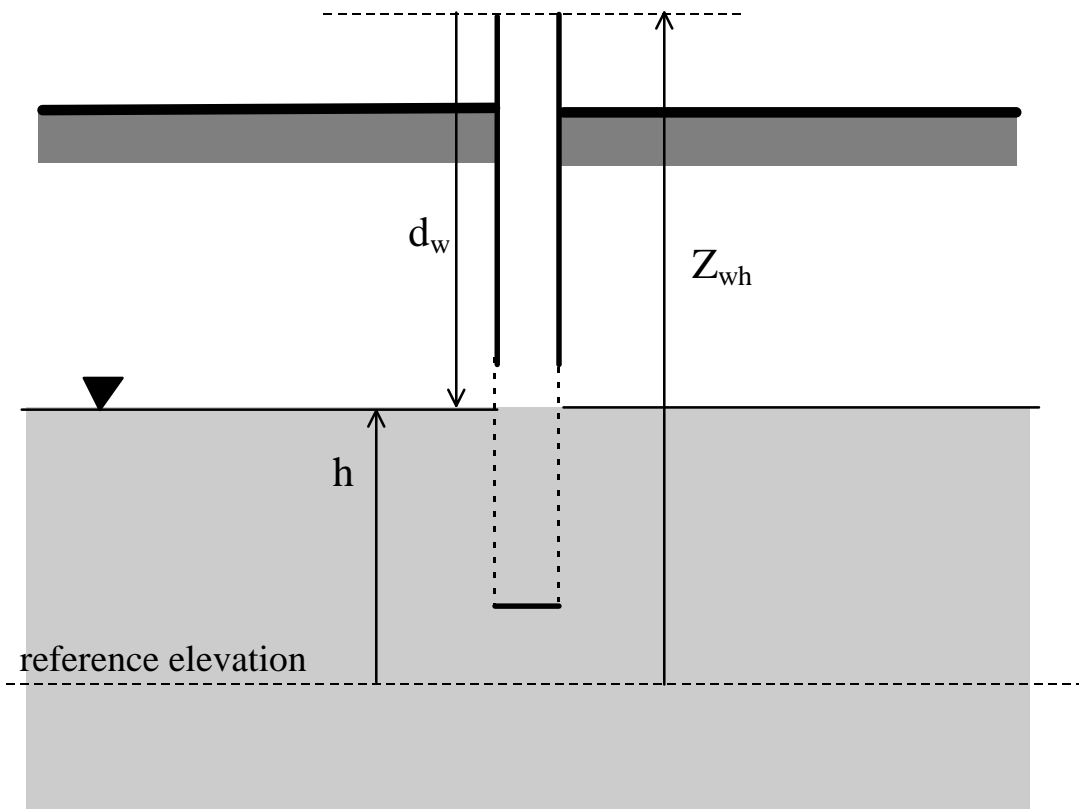


## FLOW NETS (AH: Chapter 5.11):

- Map showing lines of equal head and direction of flow.

### 1. MEASURING HYDRAULIC HEAD:

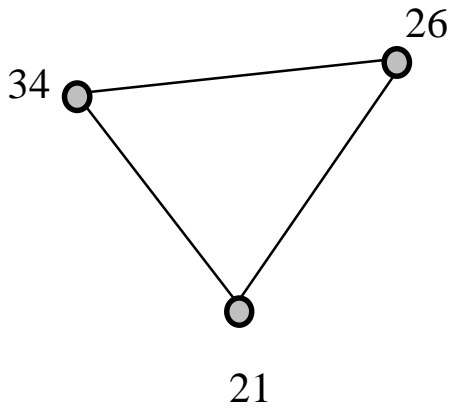


Head = Elevation at well head - depth to water

Important Considerations:

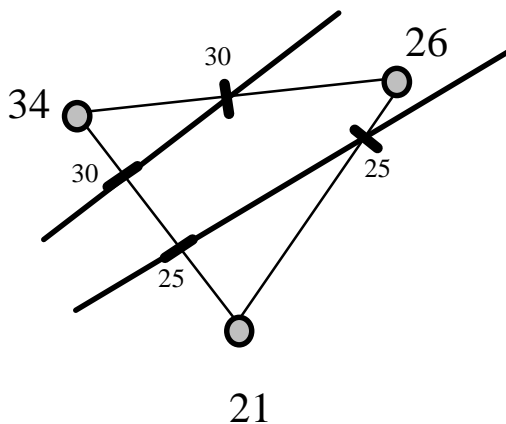
1. Screened interval of well (same aquifer? vertical gradients?)
2. Time (transient conditions [e.g., recharge], tidal effects, atmospheric pressure effects) - see fig 4.10, 4.11
3. Surveyed location of well (x,y,z).

## 2. MAPPING EQUIPOTENTIAL LINES IN A HORIZONTAL PLANE.



3 locations in a horizontal plane:  
 - wells or piezometers  
 - nodes from numerical model

All represent head at a location in the same aquifer--no vertical gradients.

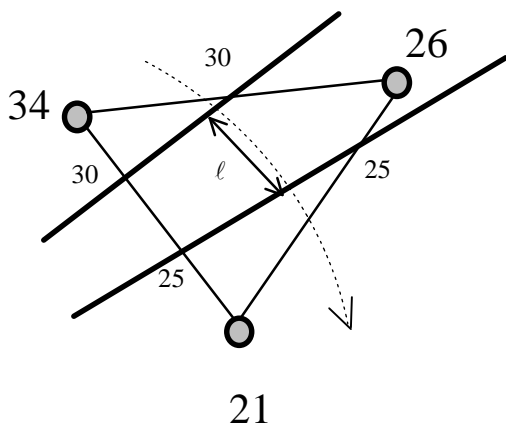


Connect all points with lines.

Interpolate head values between points.

Construct lines of equal head:

- Piezometric Surface
- Potentiometric Surface
- Water table elevation (surface aquifer only)



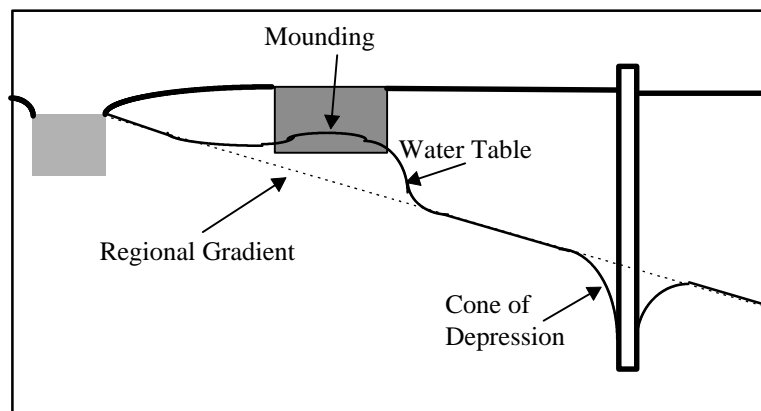
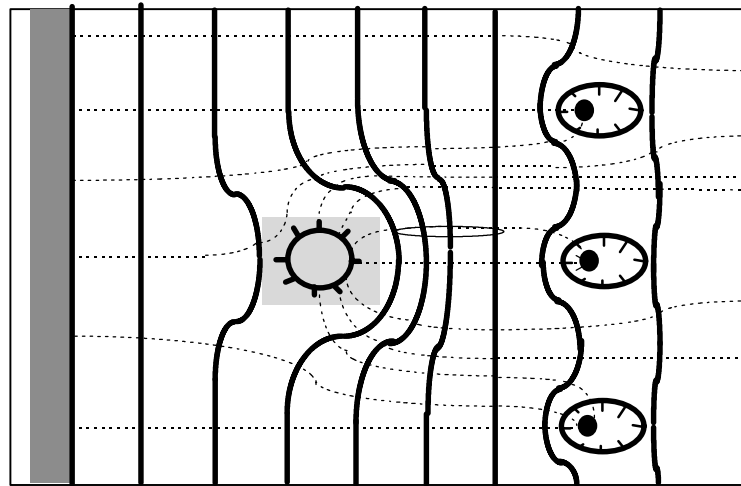
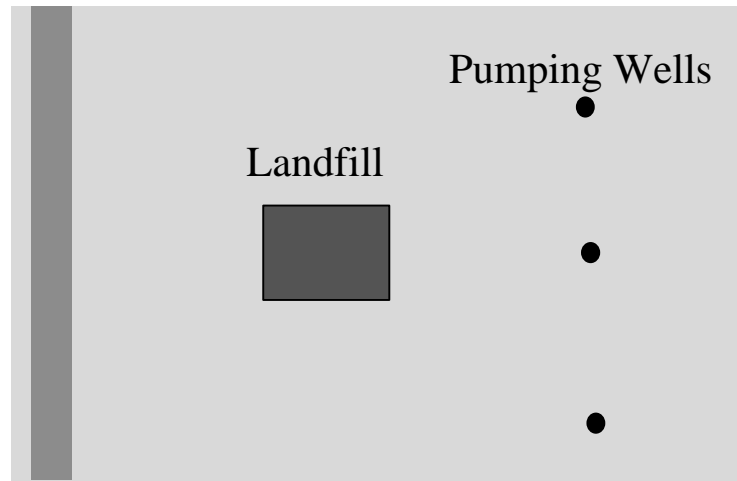
Construct flow lines crossing perpendicular to equipotential lines.

Calculate gradient along flow line,  
 $(h_2 - h_1)/\ell$ .

Apply Darcy Equation to calculate velocity.

## FLOW NETS: Continued:

River



Observe patterns in flow net:

1. Equipotential lines closer together = steeper gradient  
= increasing flow rate?  
= decreasing hydraulic conductivity?
2. Flow lines: Converging = increasing flow rate  
Diverging = decreasing flow rate

Demonstrate velocity vectors

Demonstrate solute transport path

---

### **$\Delta$ Storage Term**

Affected by 3 variables which may change with TIME:

Note:  $z \neq f(t)$

1. Water Density  
Compressible Fluid,  $\rho = f(P) = g(h)$   
Incompressible Fluid,  $\rho = \text{constant}$
2. Porosity  
Deformable Media,  $n = f(P) = g(h)$   
Nondeformable Media,  $n = \text{constant}$
3. Saturation  
Unsaturated,  $S = f(P) = g(h)$   
Saturated,  $S = 1$

## $\Delta$ Storage Term

8 possible combinations - We will discuss 3:

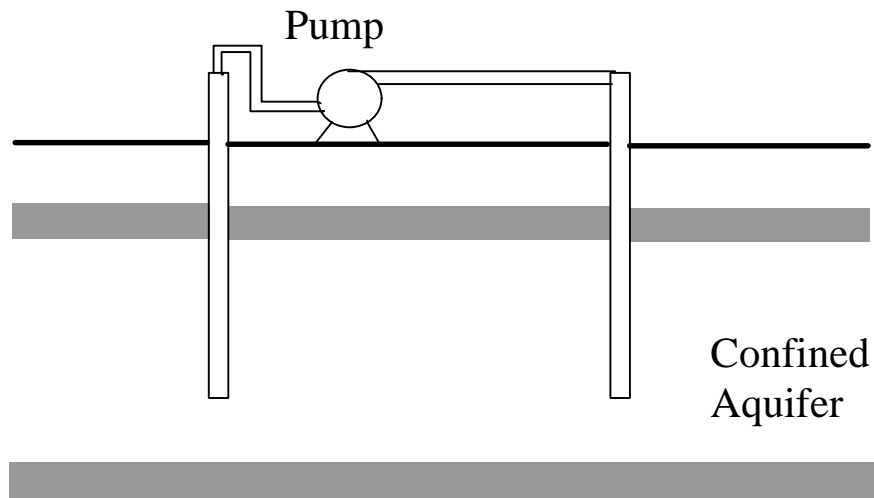
*CONDITION 1: Incompressible fluid, nondeformable media, saturated conditions:*

$$\frac{\partial}{\partial t}(\rho\theta) = 0$$

Condition required for steady state.

Does it guarantee steady state?

What is steady state?



$\Delta$  **Storage** = **0** does not mean that flow rates, flow directions, and hydraulic heads cannot change with time.

## **Δ Storage Term, Continued:**

*CONDITION 2: Compressible fluid, deformable media, saturated conditions:*

## **Specific Storage Relationship**

$$\frac{\partial}{\partial t}(\rho n) = \rho S_s \frac{\partial h}{\partial t}$$

Consider: Confined aquifer under pressure. Relieve pressure by pumping:

1. Water expands slightly; density decreases.
2. Soil grains “settle” or compress; porosity decreases.

Effects are combined in specific storage,  $S_s$ , ( $L^{-1}$ )

$$S_s = \frac{\text{Vol. H}_2\text{O released from storage}}{(\text{Vol. Aquifer})(\text{Unit decline in head})} \quad \text{Typical value} \approx 10^{-6} \text{ ft}^{-1}$$

Note: density is assumed not be a function of space, and drops from the mass balance equation.

Insert in Flow Balance Equation:

Heterogeneous/Anisotropic:

$$S_s \frac{\partial h}{\partial t} = \frac{\partial}{\partial x} K_x \frac{\partial h}{\partial x} + \frac{\partial}{\partial y} K_y \frac{\partial h}{\partial y} + \frac{\partial}{\partial z} K_z \frac{\partial h}{\partial z}$$

Homogeneous/Isotropic:

$$\frac{S_s}{K} \frac{\partial h}{\partial t} = \frac{\partial^2 h}{\partial x^2} + \frac{\partial^2 h}{\partial y^2} + \frac{\partial^2 h}{\partial z^2}$$

*CONDITION 3: Incompressible Fluid, Nondeformable Medium, Unsaturated Conditions.*

To be covered in Unsaturated flow unit.

---

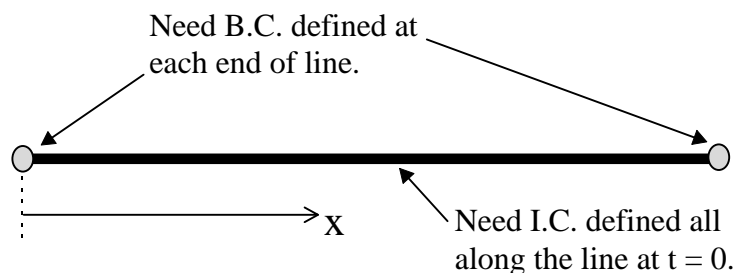
## BOUNDARY CONDITIONS:

Mass balance equation lets us develop the GOVERNING EQUATIONS for flow in porous media.

To model the system (i.e., solve the equations), we need to know the boundary conditions:

1. *Boundary conditions in space:*  
Needed for both transient and steady state solutions.
2. *Boundary conditions in time (a.k.a. initial conditions):*  
Needed for transient solutions.

1-D Problem:



## BOUNDARY CONDITIONS:

### 2-D Problem:



### TYPES OF BOUNDARY CONDITIONS:

- Flow equations are solved in terms of head,  $h$
- Boundary conditions must be defined in terms of head.

1. Constant head boundary:  $h(x=0, y, t) = 20 \text{ ft.}$

#### Examples:

- Surface water (river, lake) with “constant” elevation
- Regional groundwater elevation at a location beyond influence of local effects (e.g., pumping)

2. Constant flux boundary:  $\left. \frac{\partial h}{\partial y} \right|_{(x, y=0, t)} = 0$

Constant gradient = constant flow rate

zero gradient = no flow

#### Examples:

- Boundary parallel to mean flow direction beyond range of local effects.



### 3. Initial Conditions: $h(x,y,t=0) = f(x,y)$

- Can use steady state solution for flow conditions prior to application of stresses which cause transient conditions.

Questions:

Are boundary/initial conditions reasonable assumptions?

Do the boundary conditions remain reasonable throughout the simulation?